

**DYNAMIC CONE PENETRATION TEST
REPORT
TEMPORARY OFFICE SHED
MFMRD**

Prepared by:

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04 JANUARY 2024

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1.0 Introduction

This report represents the result of a Dynamic Cone Penetration Test (DCPT) undertaken for a proposed Temporary Office Shed for the Ministry of Fisheries & Marine Resource Development (MFMRD). The investigation was commissioned in an email dated 19th December 2023 by the MFMRD and was undertaken in accordance with the Australian Standard AS-1289 Methods of Testing Soils for Engineering Purposes by qualified engineers from Mylanta Service.

The aim of the investigation was to assess the subsurface soil and groundwater condition across the site in order to provide comments and recommendation on:

- The geotechnical suitability of the site for the proposed office.
- The depth of groundwater, if encountered.
- Site preparation, compaction, earthworks, and remediation, if required, so as to allow the proposed development.
- Appropriate foundation system(s) for the proposed structure.
- Foundation design parameters including an indicative California Bearing Ratio, (CBR) value based on field testing, including the allowable bearing pressure, (ABP) of the site at foundation depth.

2.0 Investigation

2.1 Methodology

The investigation includes a site walkover to observe the topographic setting within which the site is located, 32 identified spots of dynamic cone penetration test, (DCPT) to determine the allowable bearing capacity of the proposed site. 9 of which, spot 21-29, was not tested due to the obstruction of an existing old building

The purpose of the DCPT was to measure the equivalent penetration index values in the subsoil units in order to determine the penetration resistance of the subsoil, so as to calculate the CBR and the ABP. In this test, a 1000mm length of 16mm diameter steel rod with a 20mm diameter cone tip is driven into the ground using a 9kg hammer with a drop height of 510mm. Blow counts were recorded as the whole number of blows for successive 100mm penetrations in accordance with AS 1289 method 6.3.2.

The investigation was undertaken on the 30th of December 2023 and 2nd of January 2024 by Mr Kaiea Collins and associates from Mylanta Services.

2.2 Result

2.2.1 Site Description

The proposed site is located at Ambo Village, Tarawa. The site is an open field with few vegetation adjacent to a telecommunication tower and an existing old building situated within the proposed structure footprints.

Groundwater depth was not found during the test and is however identified to be lower than 1m.

The site's topography is flat surfaced with low field grasses, shrubs and coconut trees. Ground penetration resistance generally vary and are described in section 2.2.2.

2.2.2 Ground Penetration Resistance

The ground penetration resistance is given in DCPT results the as in Appendix 2. The table illustrations are depths with blows/100mm and their corresponding correlation to CBR, and ABP.

The allowable bearing pressure of the identified weak layers range from 40-80 kilopascal, (kPa), whilst the strongest layers can come up to and estimated ABP of 180 kPa. But our concern are the underlying weak layers of soil which will support the building.

3.0 Comments & Recommendations

3.1 Site Suitability

The minority of the ground conditions are generally considered suitable for the proposed design, which will not impose any significant geotechnical constraints. However, the weak subsoil layers encountered at Spot # 3-6, 11-15, 19 and 30 will require densification to form suitable subgrade material for the proposed design.

Therefore, from a geotechnical standpoint, the land is physically capable of development for the proposed Temporary Office Shed provided that the provisions outlined in the subsequent subsections of the report are taken into consideration, and the recommendations implemented.

3.2 Groundwater Level

The depth of ground water at the time of investigations was lower than 1m. The time and date of testing was ~2.00pm, 30th of December 2023 and 2nd of January 2024. And 2:00pm is low tide on the Island according to the Tide Calendar from the MET. Thus, the groundwater pressure/ hydro-static uplift has no significant impact on the proposed foundation design.

3.3 Site Preparation

All deleterious material, including topsoil and vegetation should be stripped from building envelopes. Up to 7 numbers of coconut trees were identified to be stripped. Topsoil could be re-used for landscaping purposes only.

Tree roots remaining from any clearing operations within the proposed building envelopes should be completely removed and the excavation backfilled with material of similar geotechnical properties to the surrounding ground and compacted to achieve a penetration index equal to or greater than three (≥ 3) blow/100 mm.

3.5 Subsoil Densification

Densification of subsoil at Spot # 3-6, 11-15, 19 and 30 is crucial. It is recommended that the sites are proof rolled using a heavy (minimum of 14 tonne deadweight) vibrating smooth drum roller (if available), alternatively uses vibro-compacting plates. Any areas that show signs of excessive deformation during compaction should be continually compacted until deformation ceases, or

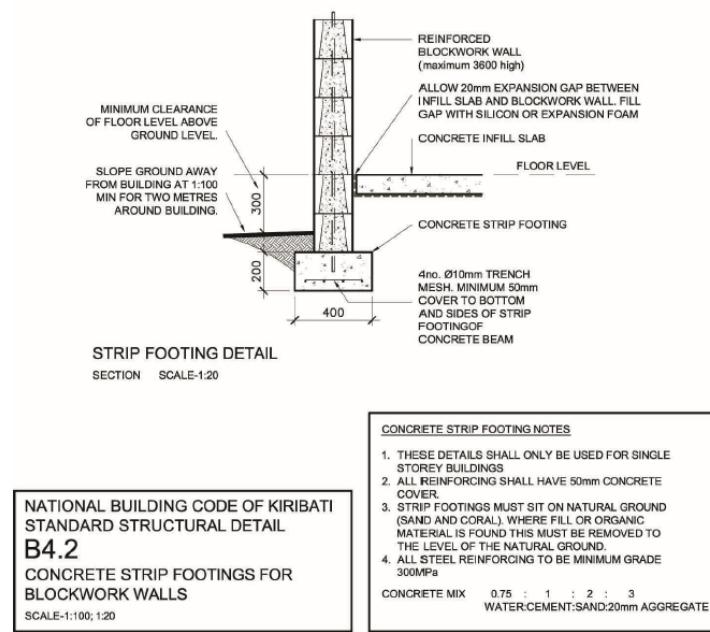
alternatively, the weak materials be removed and replaced with suitable structural filling, compacted at every 100-150 mm layer using a plate compactor until footing depth is reached.

3.4 Foundation Design

Unless the current proposed raft footing system has been analysed and designed according to AS 2870-2011, the footing shall be maintained. Otherwise, the following foundation design standard from the Kiribati National Building Code 2016, (KNBC) shall be adopted, Figure 1. This is one of the minimum shallow footing standards designed in accordance to AS 2870-201, which applies to double storey residential and small sized offices and the like classified as Class 2 under the Building Code of Kiribati.

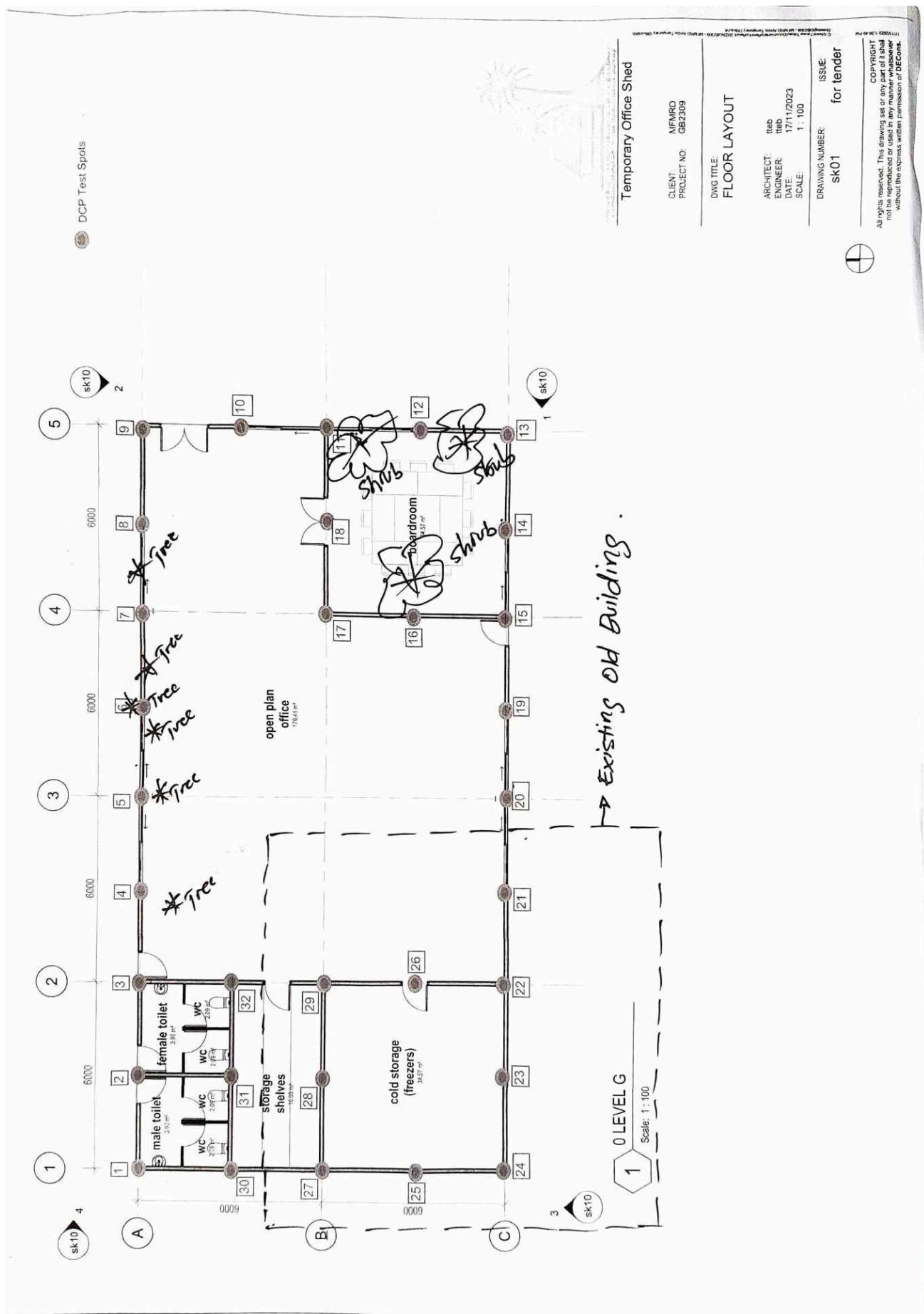
Conversely, the pier footing is 1.5 meters below the surface. In this case, it is strongly advised that a competent engineer perform a proper structural analysis and design so that the pier footing can be modified in accordance with the site's specifications.

FIGURE 1: STRIP FOOTING STANDARD STRUCTURAL DETAIL (KNBC, 2016)



APPENDIX

4.1 Appendix 1: Field Test Notes



4.2 Appendix 2: Test Results with CBR & ABP

DYNAMIC CONE PENETROMETER - RESULT SHEET AS1289.6.3.2

Client	MFMRD
Project	Temporary Office Shed
Location	Ambo
Operator	Kaiea Collins + Mylantas' Crew
Date	23/12/2023
Job #	#23-010

CBR = California Bearing Ratio
q(all) = Allowable Bearing Pressure (kPa)

Spot # 1	Spot # 2	Spot # 3	Spot # 4
Location	Location	Location	Location

Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	2	3.5	70	100	1	2	40	100	1	2	40	100	1	2	40
200	2	3.5	70	200	2	3.5	70	200	1	2	40	200	2	3.5	70
300	3	6	80	300	2	3.5	70	300	1	2	40	300	2	3.5	70
400	2	3.5	70	400	4	8	115	400	1	2	40	400	1	2	40
500	1	2	40	500	3	6	80	500	1	2	40	500	2	3.5	70
600	4	8	115	600	2	3.5	70	600	1	2	40	600	1	2	40
700	3	6	80	700	3	6	80	700	2	3.5	70	700	3	6	80
800	3	6	80	800	5	10	150	800	3	6	80	800	4	8	115
900	4	8	115	900	6	12	180	900	4	8	115	900	3	6	80
1000	3	6	80	1000	4	8	115	1000	4	8	115	1000	4	8	115

Spot # 5	Spot # 6	Spot # 7	Spot # 8
Location	Location	Location	Location

Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	1	2	40	100	2	3.5	70	100	1	2	40	100	2	3.5	70
200	2	3.5	70	200	2	3.5	70	200	1	2	40	200	4	8	115
300	2	3.5	70	300	3	6	80	300	1	2	40	300	3	6	80
400	2	3.5	70	400	1	2	40	400	1	2	40	400	3	6	80
500	2	3.5	70	500	2	3.5	70	500	1	2	40	500	4	8	115
600	2	3.5	70	600	2	3.5	70	600	1	2	40	600	4	8	115
700	3	6	80	700	3	6	80	700	3	6	80	700	4	8	115
800	3	6	80	800	4	8	115	800	6	12	180	800	5	10	150
900	4	8	115	900	6	12	180	900	6	12	180	900	4	8	115
1000	5	10	150	1000	5	10	150	1000	5	10	150	1000	3	6	80

Spot # 9	Spot # 10	Spot # 11	Spot # 12
Location	Location	Location	Location

Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	1	2	40	100	3	6	80	100	1	2	40	100	1	2	40
200	2	3.5	70	200	3	6	80	200	1	2	40	200	1	2	40
300	3	6	80	300	4	8	115	300	2	3.5	70	300	1	2	40
400	4	8	115	400	4	8	115	400	2	3.5	70	400	1	2	40
500	4	8	115	500	3	6	80	500	1	2	40	500	1	2	40
600	4	8	115	600	2	3.5	70	600	2	3.5	70	600	1	2	40
700	4	8	115	700	3	6	80	700	2	3.5	70	700	1	2	40
800	5	10	150	800	5	10	150	800	1	2	40	800	1	2	40
900	6	12	180	900	5	10	150	900	3	6	80	900	1	2	40
1000	4	8	115	1000	4	8	115	1000	2	3.5	70	1000	1	2	40

Spot # 13	Spot # 14	Spot # 15	Spot # 16
Location	Location	Location	Location

Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)	Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	1	2	40	100	1	2	40	100	2	3.5	70	100	2	3.5	70

200	1	2	40	200	1	2	40	200	3	6	80	200	2	3.5	70
300	1	2	40	300	2	3.5	70	300	2	3.5	70	300	3	6	80
400	1	2	40	400	1	2	40	400	2	3.5	70	400	3	6	80
500	1	2	40	500	2	3.5	70	500	2	3.5	70	500	2	3.5	70
600	1	2	40	600	1	2	40	600	1	2	40	600	3	6	80
700	1	2	40	700	2	3.5	70	700	1	2	40	700	2	3.5	70
800	2	3.5	70	800	2	3.5	70	800	2	3.5	70	800	3	6	80
900	2	3.5	70	900	3	6	80	900	2	3.5	70	900	2	3.5	70
1000	3	6	80	1000	4	8	115	1000	4	8	115	1000	3	6	80

Spot #	17
Location	

Spot #	18
Location	

Spot #	19
Location	

Spot #	20
Location	

Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	2	3.5	70
200	3	6	80
300	3	6	80
400	3	6	80
500	3	6	80
600	3	6	80
700	4	8	115
800	2	3.5	70
900	2	3.5	70
1000	3	6	80

Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	2	3.5	70
200	2	3.5	70
300	4	8	115
400	3	6	80
500	2	3.5	70
600	2	3.5	70
700	3	6	80
800	2	3.5	70
900	1	2	40
1000	2	3.5	70

Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	1	2	40
200	1	2	40
300	1	2	40
400	1	2	40
500	1	2	40
600	1	2	40
700	4	8	115
800	2	3.5	70
900	2	3.5	70
1000	3	6	80

Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	1	2	40
200	2	3.5	70
300	2	3.5	70
400	3	6	80
500	3	6	80
600	3	6	80
700	4	8	115
800	2	3.5	70
900	1	2	40
1000	2	3.5	70

Spot #	30
Location	

Spot #	31
Location	

Spot #	32
Location	

Spot #	33
Location	

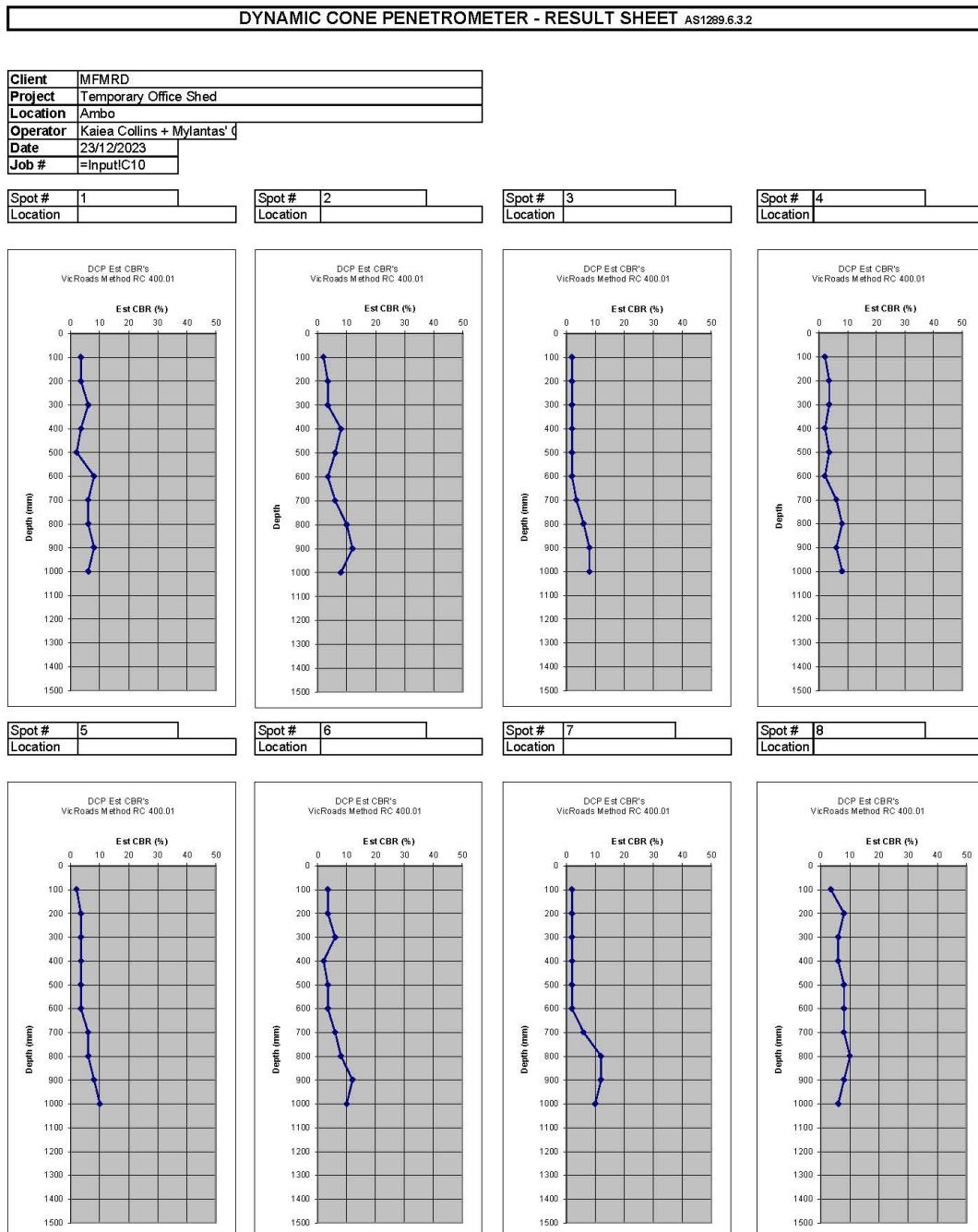
Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	1	2	40
200	1	2	40
300	1	2	40
400	1	2	40
500	1	2	40
600	1	2	40
700	1	2	40
800	2	3.5	70
900	4	8	115
1000	5	10	150

Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	1	2	40
200	2	3.5	70
300	3	6	80
400	2	3.5	70
500	3	6	80
600	3	6	80
700	4	8	115
800	4	8	115
900	6	12	180
1000	4	8	115

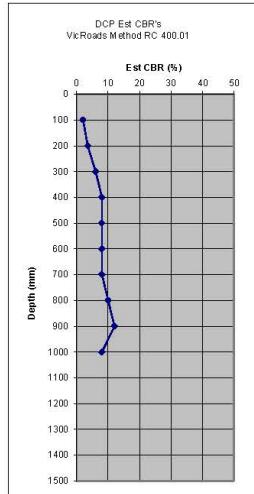
Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100	3	6	80
200	2	3.5	70
300	3	6	80
400	2	3.5	70
500	3	6	80
600	3	6	80
700	2	3.5	70
800	2	3.5	70
900	2	3.5	70
1000	3	6	80

Depth mm	Blows / 100 mm	Est CBR	Est q(all)
100		#N/A	#N/A
200		#N/A	#N/A
300		#N/A	#N/A
400		#N/A	#N/A
500		#N/A	#N/A
600		#N/A	#N/A
700		#N/A	#N/A
800		#N/A	#N/A
900		#N/A	#N/A
1000		#N/A	#N/A

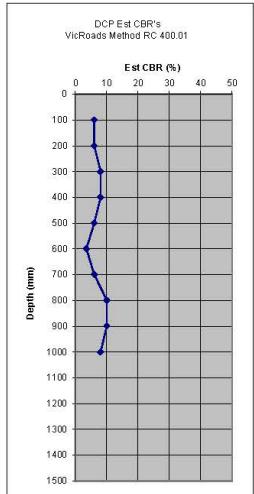
4.3 Appendix 3: CBR & ABP Graphs



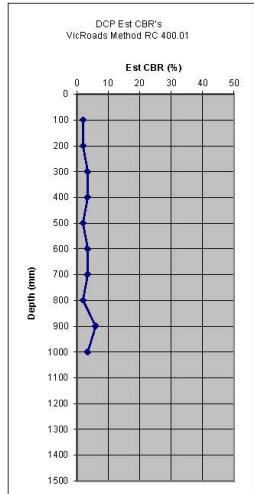
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Location



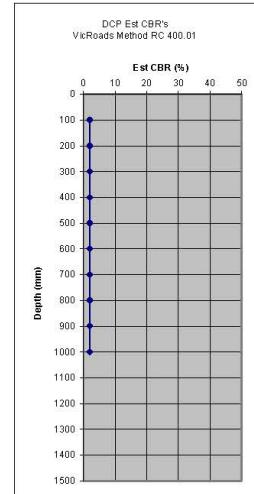
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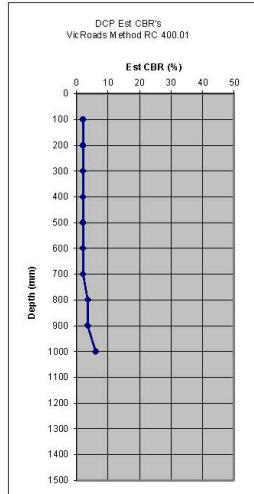
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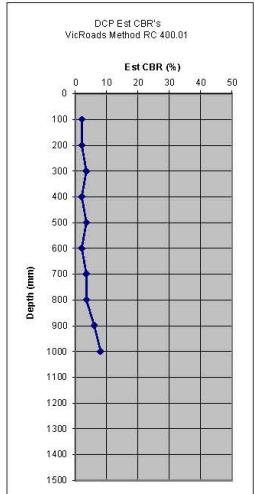
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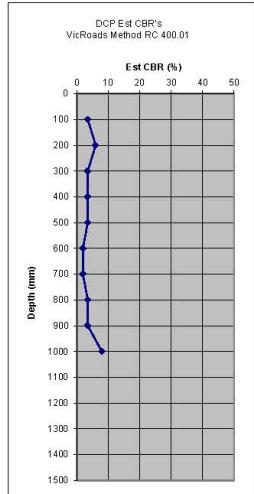
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Location



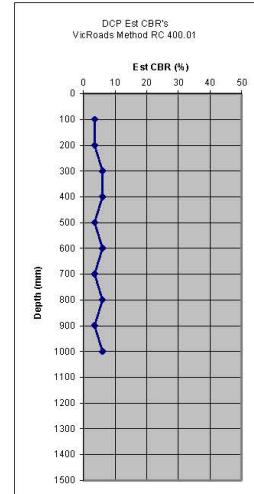
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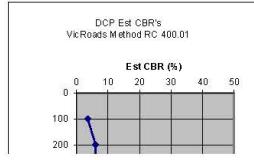
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Location



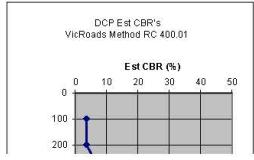
Spot # 16
Location



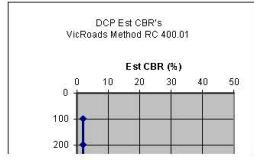
Spot # 17
Location



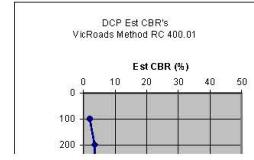
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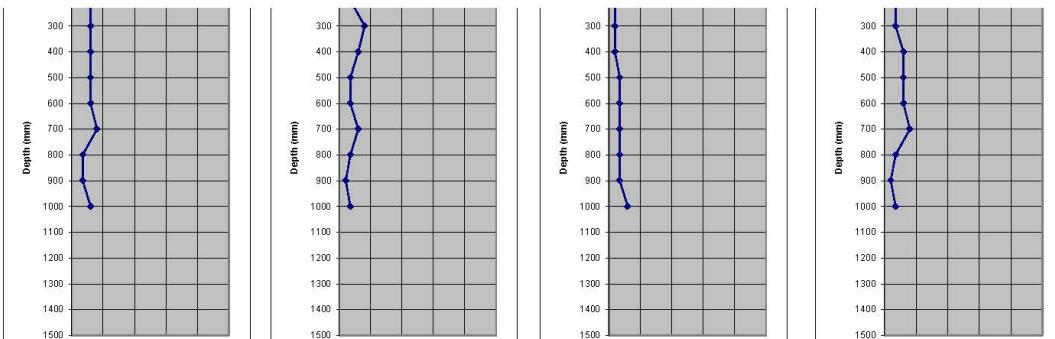


Spot # 19
Location



Spot # 20
Location



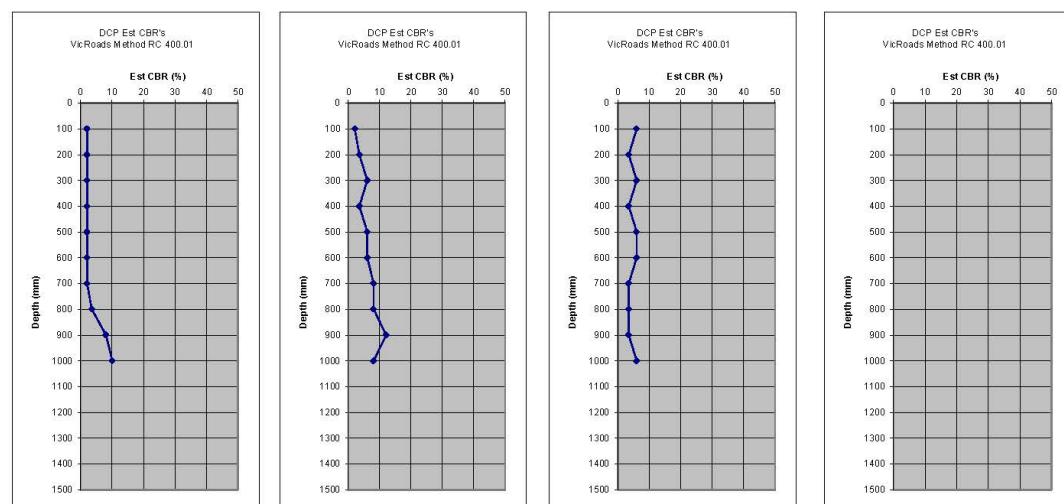


Spot # 30
Location

Spot # 31
Location

Spot # 32
Location

Spot # 33
Location



DYNAMIC CONE PENETROMETER - RESULT SHEET AS1289.6.3.2

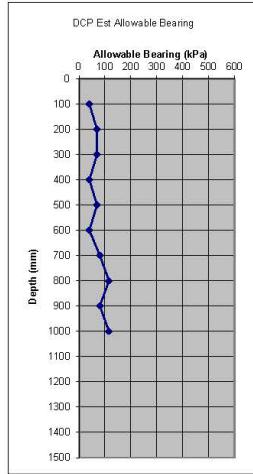
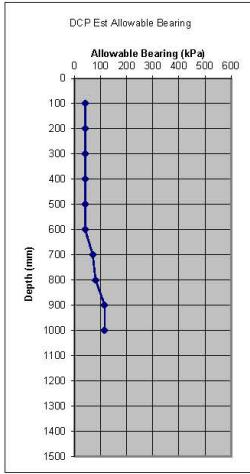
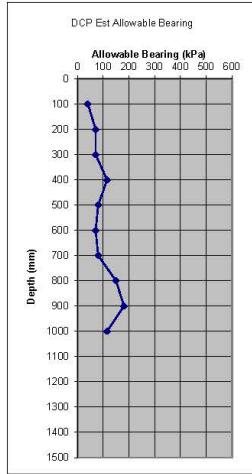
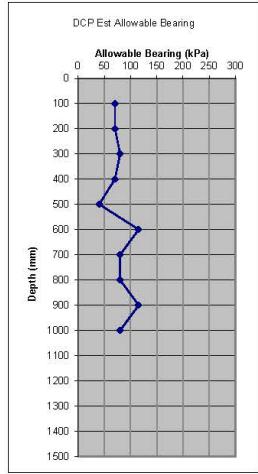
Client	MFMRD
Project	Temporary Office Shed
Location	Ambo
Operator	Kaiea Collins + Mylantas'
Date	23/12/2023
Job #	=Input!C10

Spot # 1
Location

Spot # 2
Location

Spot # 3
Location

Spot # 4
Location

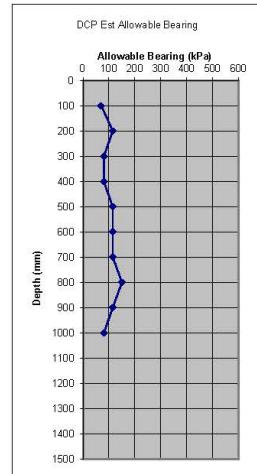
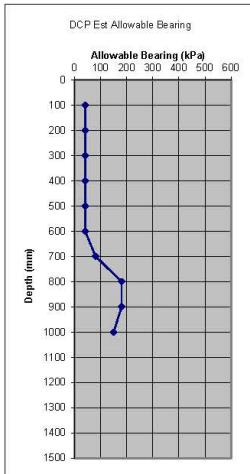
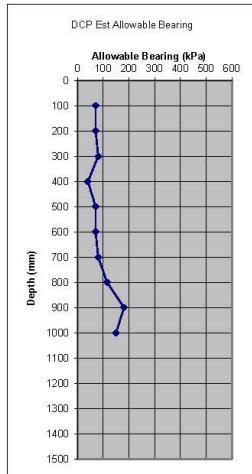
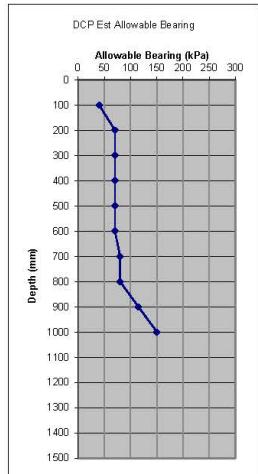


Spot # 5
Location

Spot # 6
Location

Spot # 7
Location

Spot # 8
Location

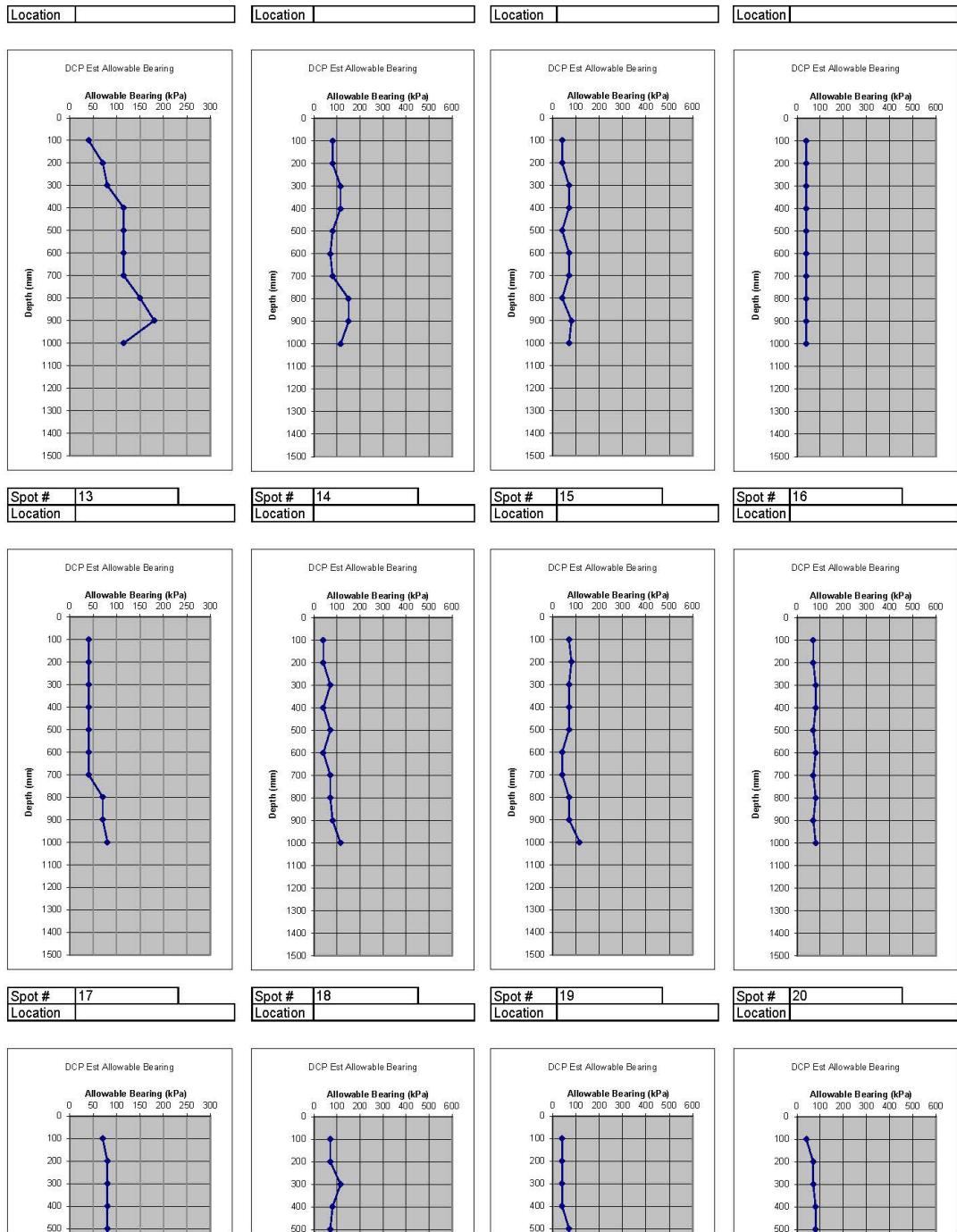


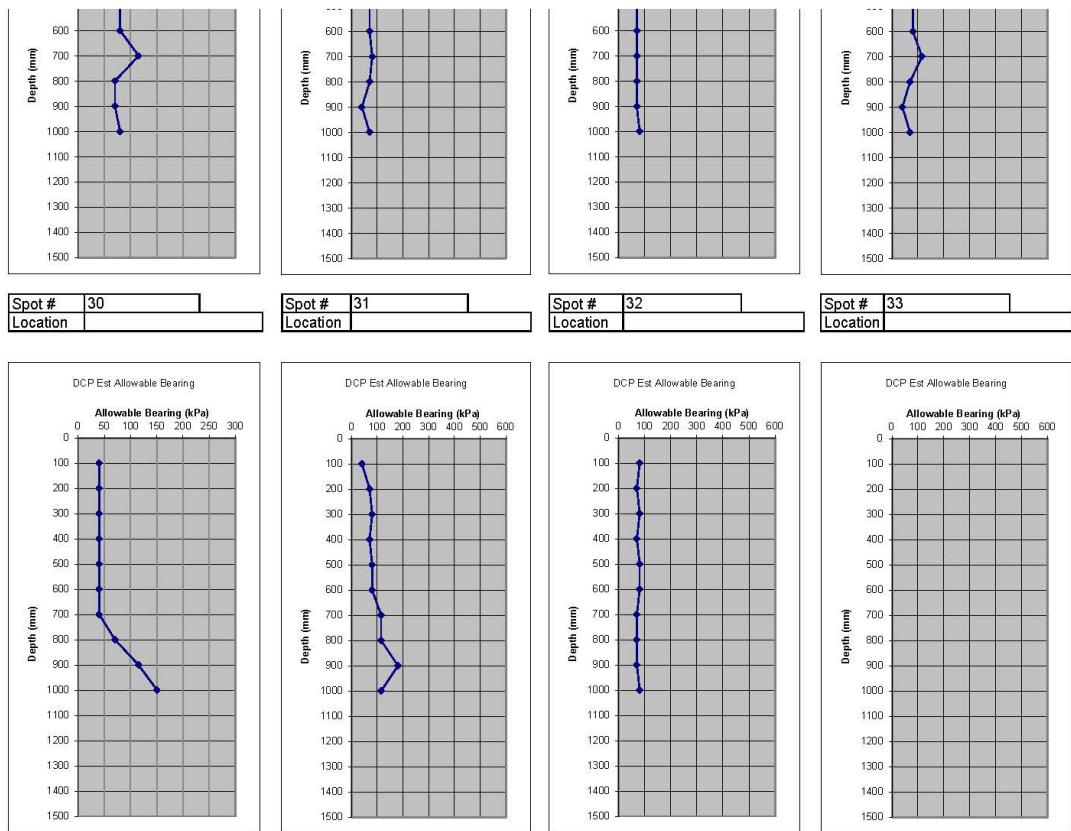
Spot # 9

Spot # 10

Spot # 11

Spot # 12





4.4 Appendix 4: Site Photographs

